### introduction to DNV-RP-F105 Free Spanning Pipelines

### Introduction

presented by

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DNV RP-F105 Free Spanning Pipelines

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# **Historical Perspective - VIV**

	• Maximum allo Implicitly ass Do not accourt	owable span length umes natural frequency f <sub>0</sub> controlled b nt for free span scenario, loading pher	by free span length nomenon or enviror	nment					
70ties	• Fatigue Criter	ia for In-line							
	True ULS acc Arbitrary mod	counting for stress amplitude and num dels and SN-curves applied. Effect of	ber of cycles (η=0. waves?	.1)					
80ties	• Onset criteria	for Cross-flow							
	Cross-flow V Do not accourt	IV not allowed. OK for "short" spans nt for stress ranges and time to failure	and current conditation if exceeded.	ions					
1998	• Fatigue Criter	ria for Cross-flow							
	True FLS accounting for stress amplitude and number of cycles								
	Provides robu	ist decision criteria.							
,	Other failure	modes may be governing (in-line fatig	gue, over-stress)	ĴÅ					
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# **Historical Perspective - VIV**





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## Free Span Assessment -Multidiscipline

- Environmental conditions
  - Flow conditions from combined wave and current
  - Local topography
- Loading Mechanism
  - Vortex Induced Vibration (in-line & cross-flow)
  - Direct wave loads & Proximity Effects
- Structural Response
  - Soil-pipe interaction
  - Non-linearities (geometrical, static/dynamic properties)
- Acceptance criteria
  - SN-approach (weld, defects, ...)



## Basis for GL14/DNV-RP-F105

VIV Models based on experience from R&D projects & pipeline design

- MULTISPAN Project (1994-1996)
  - Response Model for In-line VIV
  - On-set criteria for cross-flow
  - Reliability based calibration

### • GUDESP PROJECT (1989-1994)

- Cross-flow Response model
- Effect of Waves
- Research projects
  - SVS full scale test
  - MASPUS lab test
- DHI/Statoil study

- Allows for state-of-the-art fatigue analyses
- Links in-line VIV and wave loads
- Allows cross-flow vibrations
- Safety philosophy in compliance with DNV-OS-F101
- Introduces consistent link between analysis models and safety factor(s)
- Applied in numerous projects in
  - North Sea
  - Persian Gulf
  - South East Asia
  - GOM



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## DNV GL 14 $\rightarrow$ DNV-RP-F105 - why update?

- Include experience feed-back from projects
- Include recent R&D effort:
  - Pipe in trench
  - VIV response model updates
  - Hydrodynamical coefficients
  - Structural response estimates
  - Soil stiffness
  - Force model (frequency domain)
  - Recommended SN curves
- Make it more user-friendly:
  - screening (on-set) criterion
  - make criteria and calculation methods more complete
  - restructure document



## Failure Modes

### Fatigue Limit State

- .. accumulated damage from stress cycles caused by:
- Vortex Induced Vibrations (in-line & cross-flow) (RP-F105)
- Direct Wave Loads (RP-F105)

### Ultimate Limit State

- .. over-stress (local buckling) due to:
- Static Bending (weight & current) (DNV OS-F101)
- VIV & Wave Loads (RP-F105)
- Pressure Effects (DNV OS-F101)
- Axial Force (DNV OS-F101)
- Trawl interference (GL 13)



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## **Calculation Tool**

- Free span assessment complex
- Require detailed knowledge in several disciplines:
  - hydrodynamics, VIV and load models
  - environmental conditions, long-term statistics
  - fatigue calculations
  - structural response incl. geotechnical aspects
- DNV-RP-F105 still complex (and difficult?) to use
- Need for a calculation tool to:
  - make it easier to apply the RP
  - enable a cost-efficient span assessment







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DNV RP-F105 Free Spanning Pipelines

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LEVEL 1   LEVEL 2   LEVEL 3	SPANS SPANS SPANS	OPTIC USER <u>H</u> PRINT RE	DNS ELP SULTS	FATIG DNV version	UE ANAL	FAT YSIS OF F Expiry date:	F'REE REE SPAN 2002-12-31	Current NING PIPE	Vers. 9.0 LINES Release Note		Kim Mør Olav Fyrile Dee	Program by: k (Kim.Mork@a iv (Olav.Fyrileiv p Water Techno	lnv.com ) v@dnv.com) logy
	FATFREE C	OMPLETED		Project:				Date:	2002-06-10	Calculations	by		
1	No Wave Case	e		References:	~ .	_		_	•	Verified by	-	~ ~ ~	
Calculation Options Current Modelling		Modelling	Free Span Scenario		Response Data		Damping		SN-Curves		Safety Factors		
Con		Waya	Indolling	h [m]	200	f_(in-line)	0.722	Costr Denne	0.000	m,	3	n	0.50
RP-F105	ie <b>→</b>	wave w	louening	L [m]	50	f <sub>o</sub> (cr-flow)	2.000	$\zeta_{\rm soil}$ (in-line)	0.000	m <sub>1</sub>	3	γ <sub>ι</sub> ,	1.30
Return Peri	od Values	Directi	onality	e [m]	3.00	A <sub>in</sub> (in-line)	186	$\zeta_{\text{soil}}$ (cr-flow)	0.000	$Log(C_1)$	11.630	γ <sub>f</sub>	1.20
Automatic Ge	enerated -	Discrete - C	dir	d [m]	0	A <sub>cr</sub> (cr-flow)	186	ζ <sub>h,RM</sub>	0.000	Log(C2)	11.630	γs	1.05
UPDATES	SHEET	Environm	ental Data	θ <sub>pipe</sub>	0.0	$\lambda_{max}$	484	- /		logN <sub>sw</sub>	8.00	$\gamma_{on}$	1.10
				D [m]	0.500	δ/D	1.52	K <sub>s</sub> (in-line)	0.00	S <sub>0</sub> [MPa]	0.00		
<u>C</u> ALCU	LATE	Cur	rent	L/D	100	$S_{eff}/P_E$	0.13	K <sub>s</sub> (cr-flow)	0.00	SCF	1.00		
						$L_{eff,vs}/L$		Soil st	iffness			$\Psi_{R}$	1.00
						L <sub>eff,v</sub> /L		Sand - Media	ım 🔻				
								K <sub>V</sub>	1.907E+07			🗖 Well Defin	ied Span
								KL	1.430E+07				
								K <sub>V,S</sub>	5.300E+05				
	FATIGU	J <b>E LIFE</b>		cross-flow	direction	DYNAMIC	C STRESS [	MPa] <b>in-li</b> i	ne direction	E	<b>XTREME</b> (	CONDITIO	NS
In-line (Respo	nse Model)	2.10E+00	yrs	Pea	<u>k Stress</u> <u>V</u> .	. Mises Stress	Pea	<u>k Stress</u> <u>V</u> .	Mises Stress	Cur	<u>rent</u>	Wa	ives
In-line (Force	Model)	-	yrs	σ <sub>x</sub> (1 year)	0.0	319.8	σ <sub>x</sub> (1 year)	22.0	113.7	U <sub>C</sub> (1 year)	0.76	U <sub>S</sub> (1 year)	0.00
In-line (Comb	ined)	-	yrs	σ <sub>x</sub> (10 year)	0.0	319.8	σ <sub>x</sub> (10 year)	22.0	113.7	U <sub>C</sub> (10 year)	0.76	U <sub>S</sub> (10 year)	0.00
Cross-Flow		8.33E+05	yrs	$\sigma_{\rm x}(100 \text{ year})$	0.0	319.8	$\sigma_{\rm x}(100 \text{ year})$	22.0	113.7	U <sub>C</sub> (100 year)	0.76	U <sub>s</sub> (100 year)	0.00
				20 44	tribution vs dire	ection		20 0.40	RM(cross-flow RM(inline)*10	locity			
					STR	UCTURAI	MODELL	LING		-			
Static Stress [MPa] Transfer values		er values	Area	<b>s</b> [m <sup>2</sup> ]	Function	nal Loads	Pipe Dime	nsions [m]	Cons	stants	Densitie	<b>s</b> [kg/m <sup>3</sup> ]	
$\sigma_{\rm h}$	48.0	EIsteel	1.80E+08	A <sub>i</sub>	0.16619	H <sub>eff</sub> [N]	0.00E+00	Ds	0.5000	V	0.30	$\rho_{steel}$	7850
$\sigma_{\rm N}$	6.8	m <sub>e</sub>	472	A <sub>steel</sub>	0.03016	p [bar]	60	t <sub>steel</sub>	0.0200	$\alpha [°C^{-1}]$	1.17E-05	$\rho_{concrete}$	0
$\sigma_{M,cr}$	299.9	q	670	A <sub>coating</sub>	0.00000		0	t <sub>concrete</sub>	0.0000	E [IN/m ]	2.07E+11	$\rho_{coating}$	0
$\sigma_{M,in}$ (100y)	66.6	S <sub>eff</sub>	-3.99E+05	A <sub>concrete</sub>	0.00000	Coatii	ng data	t <sub>coating</sub>	0.0000	$C_D(current)$	1.00	$\rho_{cont}$	200
		Ca	1.00	A <sub>e</sub>	0.19635	k <sub>c</sub>	0.00					$\rho_{water}$	1027
$\vdash$		CSF	0.00			f <sub>cn</sub> (MPa)	45						
		$\rho_{\rm s}/\rho$	1.34										

## Experience with DNV-RP-F105

- Slight relaxation compared to Guideline 14.
- Pipe-in-trench effect significant, relevant for free spans due to scouring.
- Effect of thick concrete coating significant.
- Updated boundary condition coefficients provides good estimates for the structural response of single free spans.
- DNV-RP-F105 allows significantly longer spans than older codes.

DNV-RP-F105 represents state-of-art in free span design and minimise the costs related to seabed correction and span intervention work.



## API RP 1111 (1999)

#### 4.4.3 Spans

The length of unsupported spans on an offshore pipeline should be controlled to avoid excessive loads or deformations in the pipeline.

#### 4.4.3.1 Span Limitation Due to Weight, Pressure, and Temperature

Refer to 4.1.4 and 4.6.3 for the static loads and limits on combined loads in determining the span limitation due to its own weight, pressures, temperature, and primary longitudinal loading.

#### 4.4.3.2 Span Limitation Due to Vortex Shedding

**4.4.3.2.1** Spans exposed to transverse flow of seawater due to currents and waves are subject to a phenomenon commonly referred to as *vortex shedding*. This can cause the pipeline to oscillate as vortices alternately change the pressure above it and the pressure below it as they form and detach. Large amplitude oscillations may occur unless the natural frequency of the span is sufficiently greater than the frequency of vortex shedding.

#### M = approximate mass of pipe plus mass of water displaced by pipe, in kg/m (slugs/ft).

**4.4.3.2.3** Comparison of frequencies obtained from these calculations should indicate the tendency of a span to oscillate because of vortex shedding. As with other stability calculations, determination of may be complex.

**4.4.3.2.4** Both tension and axial stiffness affect the natural frequency. The tension and axial stiffness of the pipe may increase the natural frequency above that calculated by using equation 11. Span limitation due to vortex shedding should be based on the increased natural frequency due to the combined effect of tension and axial stiffness. Alternative methods such as finite element analysis can be employed to estimate structural response to the vortex shedding. More discussion on this subject can be found in the MIT thesis<sup>8</sup> and the DNV Guide-line No. 14.<sup>5</sup>

#### 4.5 FATIGUE ANALYSIS

**4.5.1** All pipeline components such as risers, unsupported free spans, welds, J-lay collars, buckle arrestors, and flexjoints, should be assessed for fatigue. Potential cyclic loading that can cause fatigue damage includes vortex-induced-

### DNV GL14 $\rightarrow$ Updated and released as DNV-RP-F105



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