



Estimating Structural Response to Random Vibration: Reaction Forces

File ****** SUMMATION OF STATIC FORCES AND MOMENTS IN TH COORDINATE SYSTEM *****		<u>×</u>
(Spectrum Analysis summation is of FX = 144.8702 FY = 0.6776154E-02 FZ = 0.1161775E-02 MX = 0.3225024E-01 MY = 637_1856		Solution (166) Solution Information Concention Deformation Force Reaction Rev User Defined Result
MZ = 63.54481	Details of "Force Reaction" E Definition	
SUMMATION POINT = 0.0000 0.0000 0.0000		
301111104 10141- 0.0000 0.0000 0.0000	Туре	Force Reaction
	Location Method	Boundary Condition
	Boundary Condition	Fixed Support
	Orientation	Solution Coordinate System
	Reference	Relative to base motion
L	Scale Factor	1 Sigma
	Probability	68.269 %
	Suppressed	No
	Options	
	Result Selection	All
	Results	
	X Axis	144.3 lbf
	Y Axis	1.2427e-002 lbf
	Z Axis	8 1055e-003 bf

1.000

1.500

0.000

0.500

Alex Grishin, Consulting Engineer

PADT, Tempe AZ



A: Modal Total Deformation Type: Total Deformation Frequency: 1634.4 Hz Unit: in 5/30/2016 4:51 PM **15.799 Max** 14.043 12.288 10.532 8.7771 7.0217 5.2662 3.5108

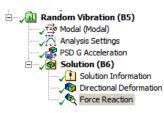
1.7554 0 Min

ANSYS Release 17.0 (January, 2016)

- Among the various enhancements, users may now estimate reaction forces directly in Mechanical (using the force reaction probe)!
 - In the past, users had to be content with inserting APDL commands (as described in:

http://www.padtinc.com/blog/the-focus/retrieving-accuratepsd-reaction-forces-in-ansys-mechanical)

 The ability to estimate accurate reaction forces for random vibration loading has existed in MAPDL since release 11 (2007)

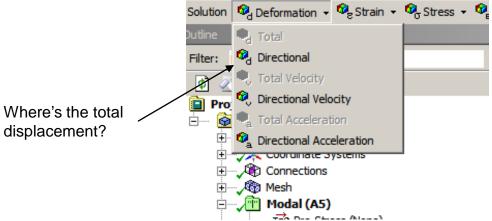


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	Definition Type Location Method Boundary Condition Orientation Reference Scale Factor Probability Suppressed Options Result Selection ResultSelection X Axis Y Axis Y Axis	

• What took so long?



- In release 17, Workbench users can still not select "Total" displacement in a random vibration environment (here again, users can invoke MAPDL commands if necessary –or even create a userdefined result)
- Again, it's reasonable to ask "why isn't this available?"





A Brief (recent) History of Random Vibration Analysis in ANSYS

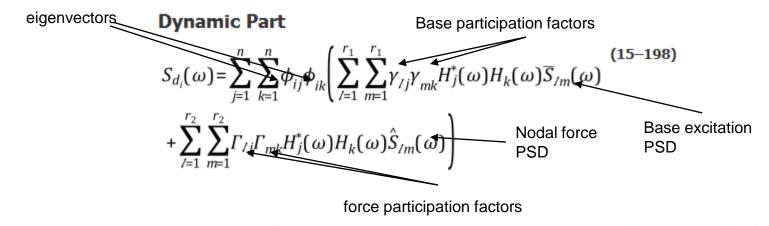
- ANSYS has had random vibration analysis capability for decades. Although PADT doesn't know precisely when it was first offered, the following are some more recent milestones in which PADT played a part:
 - August, 2001: ANSYS 5.7. PADT was made aware of 'incorrect' von Mises stress and principle stress calculations by a large aerospace customer (ANSYS 6.0 was released later that year). Customer also complains about slow solution times and large results files
 - February, 2002. PADT offers an external software 'workaround' to the problems, called x-PSD. Among the innovations, x-PSD utilizes a clever method of calculating the one-sigma von Mises stress (which we dub the 'Segalman-Reese' method after authors of the paper)
 - February, 2003. PADT stops officially developing and supporting x-PSD. Becomes a free download
 - May 2004. ANSYS releases version 8.1. Officially adopts the Segalman-Reese method for calculating one-sigma von Mises stress. Makes principle stresses unavailable in a random vibration environment
 - March, 2006. Another large aerospace customer alerts PADT that PSD reaction force calculations are 'wrong'.
 - May, 2006. International ANSYS Conference. PADT demonstrates covariance approach to calculating one-sigma reaction forces
 - June, 2007. ANSYS releases version 11, which includes new methodology to calculate reaction forces



Random Vibration Analysis: Background and Simplifications

 Users may review the 'clever' Segalman-Reese method first introduced in ANSYS 8.1 by reading the ANSYS APDL Theory manual (section 15.7.11.1). However, the procedure for estimating the reaction forces is a little more mysterious. To understand the basic problems involved, some background is necessary

Let's start with the Theory Manual. The displacement response PSD (RPSD) is:

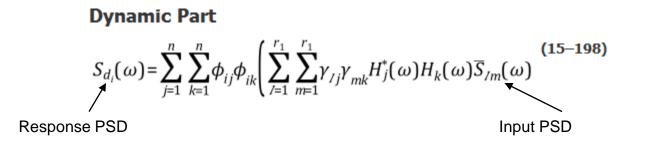




 In the previous equation, H is the complex frequency response (or 'Transfer') function*:

$$H_{j}(\omega) = \frac{1}{\omega_{j}^{2} - \omega^{2} + i(2\xi_{j}\omega_{j}\omega)}$$
(15–201)

To keep the discussion simple (without losing anything important), let's assume that we always have a single base excitation, so that (15-198) reduces to:



*We're dropping units and assuming the output PSD units are the same as the input PSD units (we're ignoring PSDUNIT and PSDRES)



• The mean square response (or *variance*), σ^2 , is equal to:

$$\sigma_{f_i}^2 = \int_0^\infty S_{d_i}(\omega) d\omega$$

 In what follows, it would be very helpful to re-write (15-204) in terms of the modal coordinate covariance matrix, C. And since we're only looking at a single base excitation, we'll eliminate some indices, and use the Einstein Summation Convention*:

$$\sigma^2 = \phi_i \phi_j C_{ij}$$
 (a-1)

(15 - 204)

where

$$Participation factor, \ g_{j} = f_{jk}e_{k}$$

$$C_{ij} = \int_{0}^{\infty} \left(\frac{\phi_{ik}e_{k}}{(\omega_{i}^{2} - \Omega^{2} + \mathbf{i}(2\omega_{i}\Omega\xi_{i}))} \right) \left(\frac{\phi_{jk}e_{k}}{(\omega_{j}^{2} - \Omega^{2} - \mathbf{i}(2\omega_{j}\Omega\xi_{j}))} \right) Sd\omega$$

* https://en.wikipedia.org/wiki/Einstein_notation



- Note that (a-1) represents a double-sum over the extracted modes. It also has a very flexible structure called a *bilinear form*. Among other things, the quantities φ_i and φ_i may be scalars, vectors, or matrices –it doesn't matter!
- It's important to realize that when creating contour plots for a particular result quantity, we are obtaining n-sigma quantities, n σ, where:

$$n\sigma = n\sqrt{\sigma^2}$$

- MAPDL stores these values on a nodal and element basis in the general postprocessor in load steps 3 through 5
- Workbench Mechanical also accesses these values



it's now possible to see what the issue is (was).

 Prior to ANSYS 8.1, if one wanted the one-sigma von Mises stress, ANSYS would first store the component stresses according to equation (a-1), where φ_i and φ_j correspond to the modal eigenstresses (mass-normalized stresses for modes i and j). It would then perform a standard von Mises calculation, such as:

$$\sigma' = \frac{1}{\sqrt{2}} \Big[(\sigma_x - \sigma_y)^2 + (\sigma_y - \sigma_z)^2 + (\sigma_z - \sigma_x)^2 + 6(\tau_{xy}^2 + \tau_{yz}^2 + \tau_{zx}^2) \Big]^{1/2}$$

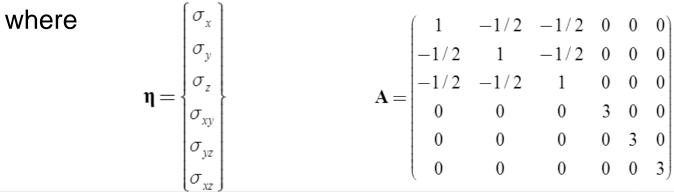
- But, since each stress component is calculated according to (a-1), it is a root-mean-square quantity. In other words, always positive!
- The issue here is that one-sigma quantities are calculated first, then the equivalent stress is calculated based on these quantities



So, what to do?

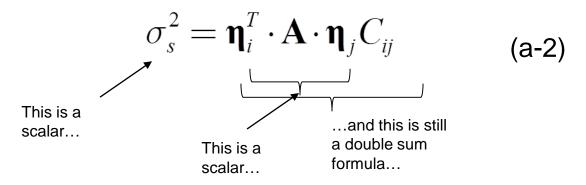
- The answer lies in first realizing that the eigenquantities in (a-1) need not be calculated on a component-wise basis, as was originally done
- Segalman and Reese realized that if they could re-write the von Mises stress as a vector product (also referred to as a *quadratic form*), then equation (a-1) could easily accommodate it. Thus, the variance (mean square) of the von Mises stress of mode j can be recaste as: 2 T

$$\sigma_{sj}^2 = \mathbf{\eta}_j^T \cdot \mathbf{A} \cdot \mathbf{\eta}_j$$





And so, (a-1) can be directly employed (and stored as a one-sigma quantity in the results file):



- This is the solution that ANSYS adopted in 2004, starting with release 8.1
- Here I'm mixing vector and indicial notation. Although a bit ambiguous, this makes the presentation much more palatable.



- So, how does ANSYS calculate the reaction forces?
- For the purpose of discussion, suppose we have nodal component of size N, and we want a vector sum of its nodal forces. The reaction force for each node in a particular direction (x, say), can be expressed as a vector in *nodal coordinates*:

$$\mathbf{f}_{x} = \begin{cases} f_{1x} \\ f_{2x} \\ \vdots \\ \vdots \\ \vdots \\ f_{Nx} \end{cases}$$

Following Segalman and Reese, the mean square of this vector can be calculated according to:

$$\sigma_{fx}^2 = \mathbf{f}_{xi}^T \mathbf{f}_{xj} C_{ij} \qquad \text{(a-3)}$$



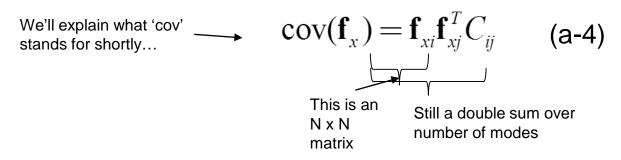
- But notice this is not the same thing as the meansquare of the SUM of the components of f (which is what we're after)!
- As we'll see shortly, prior to ANSYS release 11, FSUM produced an even worse estimate of the sum of f than would be obtained by (a-3): it simply computed each component of f according to (a-1), took the square root and summed the resulting (one-sigma) values!



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- Recall what we're after. We want the total (one-sigma) vector sum (in the x-direction in this case) of the nodal forces of the component. The mean square quantity of (a-3) does not produce this. By adding the square of each nodal force, we lose the sign, and produce a result which, in general, is way too high!
- To find the solution to this, let's do a thought experiment. Let's define a new quantity, called 'cov':



 The quantity to the left of C is a symmetric N x N matrix (the outer product of f_i and f_i instead of the dot product).

• To see how (a-4) is useful, let's expand it...

• Note first that off-diagonal terms retain their sign relative to the square-root of that row's diagonal member!



• Now, let's focus on a single row. We'll pick an arbitrary row of this matrix and divide by the square root of its diagonal term (first making sure that's not zero!). For illustration, let's take row 2. We divide each component of row 2 by f_{2x} (since that's the square roof of the diagonal term)

$$\sigma(\mathbf{f}) = \left(f_{1x}, f_{2x}, \dots, f_{Nx}\right) \quad \text{(a-5)}$$

- This gives us back the original terms! Note that it doesn't matter which row we pick. We'll always get result (a-5) as long as we divide by the square root of the diagonal term.
- But *what* have we got exactly? Well, note that the diagonal values of (a-4) are simply the mean square values (variances) of each member of **f**. It turns out that the quantity (a-4) is the *covariance* of **f**



- Dividing any row by the square root of its diagonal member should produce components whose magnitudes are each equal to their one-sigma value (by the definition of the covariance matrix), and whose sign is relative to the divisor. This allows us to produce a correct one-sigma vector SUM.
- This can be made more efficient. For example, knowing that every row of the covariance matrix will produce the same result, the calculation (a-4) thru (a-5) (and then taking the resulting sum) can be carried out on just one row. We can write this as:

$$\sigma(\Sigma f) = \frac{1}{\sigma(f_{N,N})} \mathbf{e}_N^T \cdot \mathbf{f}_i \, \mathbf{f}_j^T C_{ij} \cdot \mathbf{1} \quad \text{(a-6)}$$

Where \mathbf{e}_N^T is a unit vector containing the index of the row (N) to be kept, and 1 is an N x 1 matrix of ones*

https://en.wikipedia.org/wiki/Matrix_of_ones



- So, the main trick to finding accurate one-sigma vector sums in a random vibration environment is to first calculate the covariance matrix of the vector in question, as in (a-4)
- For instance: Suppose one needs signed one-sigma stresses (for a principle stress calculation, for example).
 All one needs is the covariance of the stress vector, η

$$\operatorname{cov}(\mathbf{\eta}) = \mathbf{\eta}_i \mathbf{\eta}_j^T C_{ij}$$
 (a-7)

This is a 6 x 6 matrix

 Still a double-sum over the number of modes...

...and then extract the signed one-sigma η :

$$\sigma(\mathbf{\eta}) = \frac{1}{\sigma(\eta_{N,N})} \mathbf{e}_N^T \cdot \mathbf{\eta}_i \mathbf{\eta}_j^T C_{ij} \quad \text{(a-8)}$$

One-Sigma stress vector



Validation by Example

- The relations (a-4) thru (a-8) are new to most engineers, but they can be easily proven from the definition of covariance matrix and arithmetic properties of the expectation operator
- Verifying these calculations numerically on complex model problems is a bit harder. In principle, this would require comparing results of these calculations to statistical properties of equivalent time histories
- However, for simple systems, we can compare these results to single DoF approximations. There are two basic techniques*:
 - 1. Miles' Equation
 - 2. Single Mode Coefficient

*We might mention in passing a third technique: One could constrain a group of nodes on which a reaction force is required to a single mass element. The node of this element will contain the correct force value



Validation by Example: Miles' Equation

 $F = M_e^* G_{rms}^* 386.4$

Modal effective mass

<u>Ryan Simmons</u> NASA Goddard Space Flight Center May 2001

Basics of Miles' Equation

The following equation is attributed to John W. Miles.

$$G_{RMS} = \sqrt{\frac{\pi}{2}} f_n Q \left[ASD_{input}\right]$$

Where:

G_{RMS} = Root Mean Square Acceleration in G's (sometimes given as ÿ_{RMS}).

$$f_n = \text{Natural frequency.}$$

$$Q = \frac{1}{2\zeta} = \text{Transmissibility (or amplification factor) at} \quad f_n$$
where ζ is the critical damping ratio.
$$[\text{ASD}_{input}] = \text{Input Acceleration Spectral Density at} \quad f_n \text{ in units of } \frac{g^2}{Hz}$$

http://femci.gsfc.nasa.gov/random/MilesEqn.html



Validation by Example: Scale mode by ANSYS Mode Coefficient

• After a random vibration analysis has been successfully performed, ANSYS allows one to extract a 'Mode Coefficient', *MC*. These values are simply the diagonals of the *Modal Coordinate Covariance Matrix*, C:

$$MC(i) = C_{ii}$$

*get,mci,mode,i,mcoef

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repeated indices... APDL for getting mode

No summation implied by

coefficient i

 To a single-DoF approximation, the ith-mode onesigma response is given by:

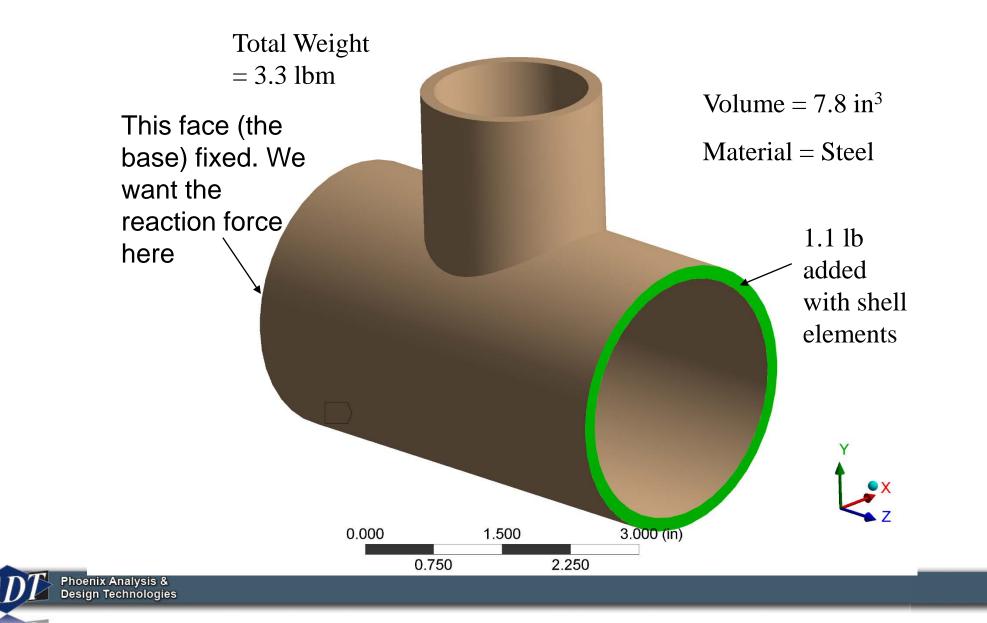
$$\sigma(\phi_i) = \phi_i \sqrt{C_{ii}}$$

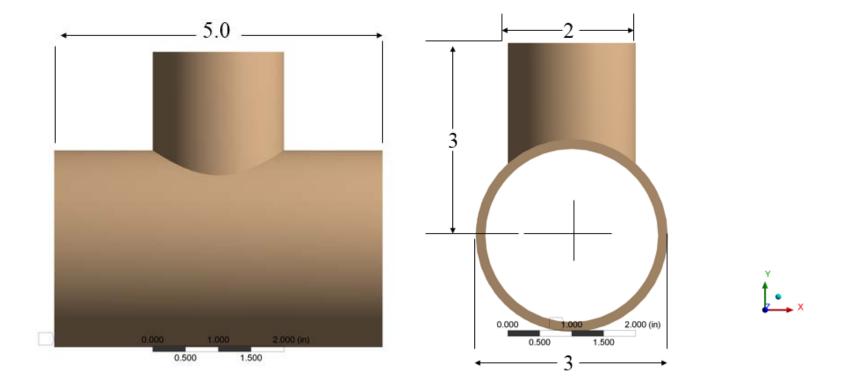
• In APDL, this can be easily achieved by scaling the ith mode by the mode coefficient:

set,1,i,sqrt(mc)

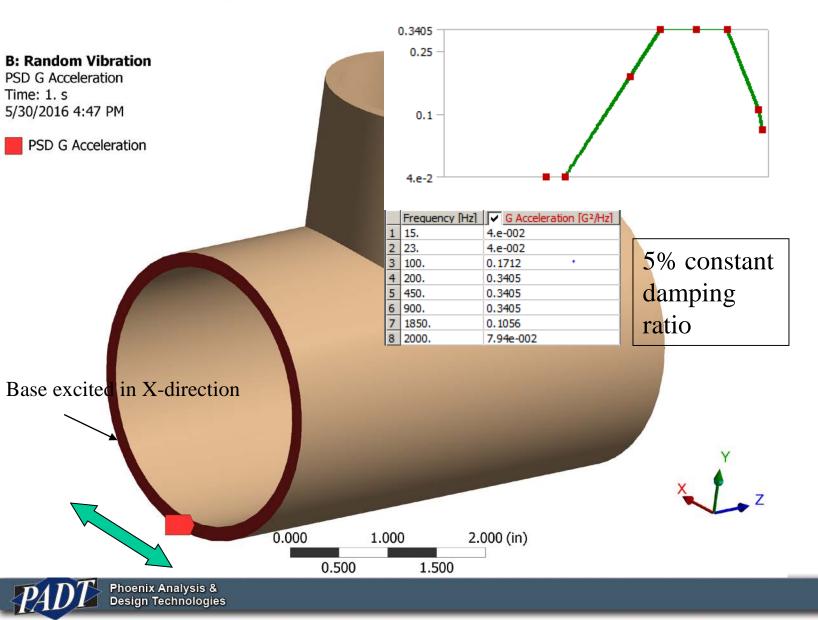
APDL for scaling ith mode by mode coefficient.

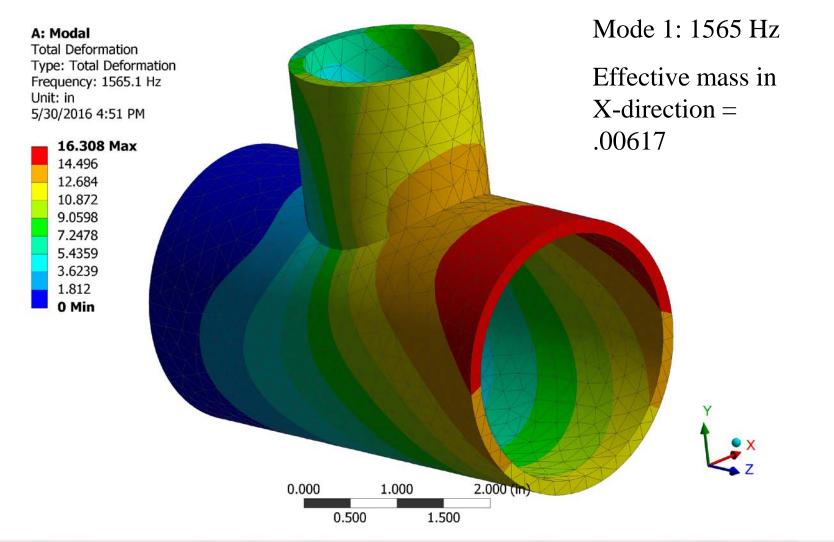
• FSUM will now produce accurate reaction force values



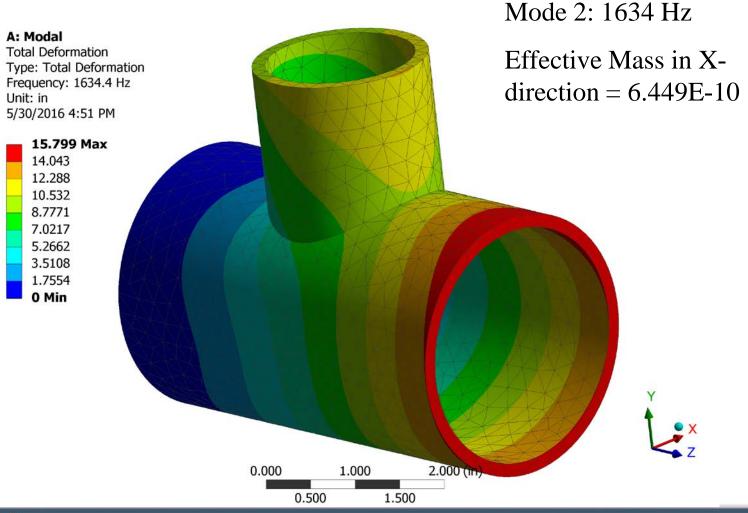




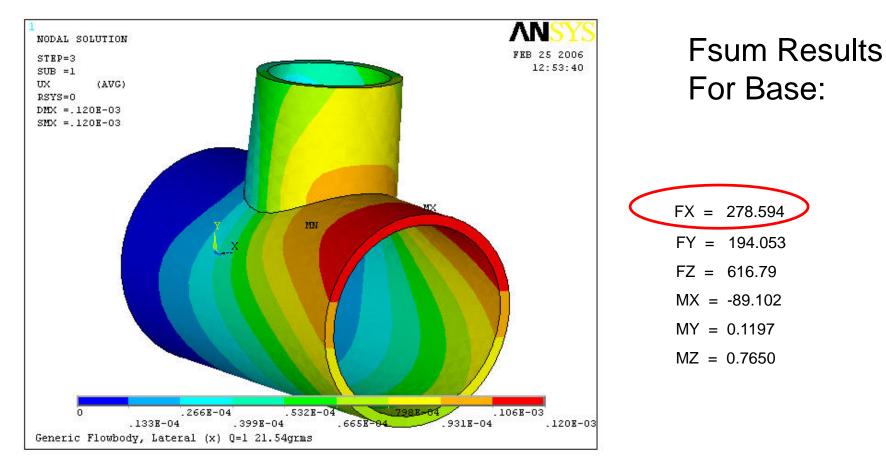












Solution 1: FSUM on Load Step 3 Prior to Release 11



Solution 2: Miles' Equation

$$G_{rms} = sqrt(pi/2*f1*Q*A)$$

$$G_{rms} = sqrt(3.14159/2*1570*10*.1378)$$

$$G_{rms} = 58.295g$$

$$F_{base} = Me*G_{rms}*386.4$$

$$F_{base} = 0.00617*58.295*386.4$$

$$F_{\rm base} = 138.98 \, \rm lbf$$

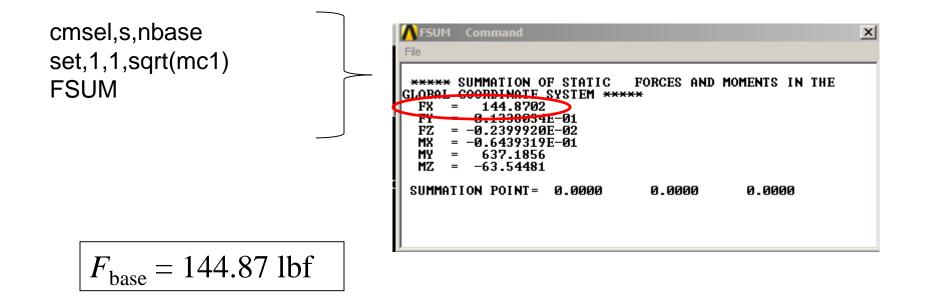
About ½ the value calculated by legacy versions (<8.1)



Solution 3: Scale Mode 1 by Mode Coefficient

MC(1) = 3.58753644E-10 ...Using *get,mc1,mode,1,mcoef

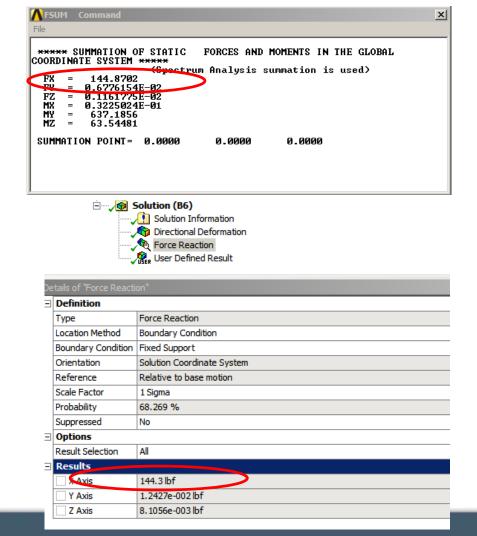
Sqrt(MC(1)) = 1.894079313E-5





Solution 4: FSUM in ANSYS releases > 11

cmsel,s,nbase set,3,1 FSUM



...And at release 17.0, this agrees (more or less) with the reaction probe in ANSYS Mechanical

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Conclusions and Summary

- For scalar engineering quantities of interest, one-sigma random quantities were always calculated according to (15-204) and (a-1)
- Historically, confusion arose over the difficulty of performing arithmetic operations on quantities calculated this way (the sign is lost, etc.). If Equations (15-204) and (a-1) were all that there was to random vibration, that would be the end of the story. However...
- This confusion evaporates once one realizes that (15-204) and (a-1) calculate the variance ONLY!
- Calculating the full covariance matrix of any result quantity allows us to recover arithmetic operations on all engineering quantities –resulting in correct one-sigma results for those quantities. ANSYS currently utilizes this epiphany for reaction forces, but no other quantities of interest (there's room for improvement)

