Direct Time-Integration Methods

These slides are based on the recommended textbook: M. Géradin and D. Rixen, "Mechanical Vibrations: Theory and Applications to Structural Dynamics," Second Edition, Wiley, John & Sons, Incorporated, ISBN-13:9780471975465



Outline

1 Stability and Accuracy of Time-Integration Operators

2 Newmark's Family of Methods

3 Explicit Time Integration Using the Central Difference Algorithm

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└─Multistep Time-Integration Methods

• Lagrange's equations of dynamic equilibrium $(\mathbf{p}(t) = \mathbf{0})$

$$\begin{aligned} \mathsf{M}\ddot{\mathsf{q}} + \mathsf{C}\dot{\mathsf{q}} + \mathsf{K}\mathsf{q} &= \mathsf{0} \\ \mathsf{q}(0) &= \mathsf{q}_0 \\ \dot{\mathsf{q}}(0) &= \dot{\mathsf{q}}_0 \end{aligned} \tag{1}$$

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First-order form

 $\underbrace{\begin{pmatrix} 0 & \mathsf{M} \\ \mathsf{M} & \mathsf{C} \end{pmatrix}}_{\mathsf{A}_{B}} \underbrace{\begin{pmatrix} \ddot{\mathsf{q}} \\ \dot{\mathsf{q}} \end{pmatrix}}_{\dot{\mathsf{u}}} + \underbrace{\begin{pmatrix} -\mathsf{M} & 0 \\ 0 & \mathsf{K} \end{pmatrix}}_{-\mathsf{A}_{A}} \underbrace{\begin{pmatrix} \dot{\mathsf{q}} \\ \mathsf{q} \end{pmatrix}}_{\mathsf{u}} = \underbrace{\begin{pmatrix} 0 \\ 0 \end{pmatrix}}_{0}$ $\Longrightarrow \underbrace{\dot{\mathsf{u}} = \mathsf{A}\mathsf{u}}_{\mathbf{u}} \quad \text{where} \quad \mathsf{A} = \mathsf{A}_{B}^{-1}\mathsf{A}_{A}$ $\blacksquare \text{ Direct time-integration}$

└─Multistep Time-Integration Methods

 \blacksquare General multistep time-integration method for first-order systems of the form $\dot{u}=Au$

$$\mathbf{u}_{n+1} = \sum_{j=1}^{m} \alpha_j \mathbf{u}_{n+1-j} - h \sum_{j=0}^{m} \beta_j \dot{\mathbf{u}}_{n+1-j}$$

where $h = t_{n+1} - t_n$ is the computational time-step, $\mathbf{u}_n = \mathbf{u}(t_n)$, and

$$\mathbf{u}_{n+1} = \left[\begin{array}{c} \dot{\mathbf{q}}_{n+1} \\ \mathbf{q}_{n+1} \end{array} \right]$$

is the state-vector calculated at t_{n+1} from the *m* preceding state vectors and their derivatives as well as the derivative of the state-vector at t_{n+1}

- $\beta_0 \neq 0$ leads to an **implicit** scheme that is, a scheme where the evaluation of \mathbf{u}_{n+1} requires the solution of a system of equations
- $\beta_0 = 0$ corresponds to an **explicit** scheme that is, a scheme where the evaluation of \mathbf{u}_{n+1} does not require the solution of any system of equations and instead can be deduced directly from the results at the previous time-steps

^LMultistep Time-Integration Methods

 General multistep integration method for first-order systems (continue)

$$\mathbf{u}_{n+1} = \sum_{j=1}^{m} \alpha_j \mathbf{u}_{n+1-j} - h \sum_{j=0}^{m} \beta_j \dot{\mathbf{u}}_{n+1-j}$$

trapezoidal rule (implicit)

$$\mathbf{u}_{n+1} = \mathbf{u}_n + \frac{h}{2}(\dot{\mathbf{u}}_n + \dot{\mathbf{u}}_{n+1}) \Rightarrow (\frac{h}{2}\mathbf{A} - \mathbf{I})\mathbf{u}_{n+1} = -\mathbf{u}_n - \frac{h}{2}\dot{\mathbf{u}}_n$$

backward Euler formula (implicit)

$$\mathbf{u}_{n+1} = \mathbf{u}_n + h\dot{\mathbf{u}}_{n+1} \Rightarrow (h\mathbf{A} - \mathbf{I})\mathbf{u}_{n+1} = -\mathbf{u}_n$$

forward Euler formula (explicit)

$$\mathbf{u}_{n+1} = \mathbf{u}_n + h\dot{\mathbf{u}}_n \Rightarrow \mathbf{u}_{n+1} = (\mathbf{I} + h\mathbf{A})\mathbf{u}_n$$

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Numerical Example: the One-Degree-of-Freedom Oscillator

Consider an undamped one-degree-of-freedom oscillator

$$\ddot{q} + \omega_0^2 q = 0$$

with $\omega_0 = \pi \operatorname{rad/s}$ and the initial displacement

$$q(0) = 1, \ \dot{q}(0) = 0$$

exact solution

$$q(t)=\cos\omega_0 t$$

associated first-order system

where

$$oldsymbol{\mathsf{A}} = \left[egin{array}{cc} 0 & -\omega_0^2 \ 1 & 0 \end{array}
ight]$$

 $\mathbf{u} = [\dot{q}, q]^{T}$, and initial condition

$$\mathbf{u}(0) = \begin{bmatrix} 0 \\ 1 \end{bmatrix}$$

Numerical Example: the One-Degree-of-Freedom Oscillator

Numerical solution

$$T = 3s, h = \frac{1}{32}$$



Stability Behavior of Numerical Solutions

- Analysis of the characteristic equation of a time-integration method

 - consider the first-order system u
 = Au
 for this problem, the general multistep method can be written as

$$\mathbf{u}_{n+1} = \sum_{j=1}^{m} \alpha_j \mathbf{u}_{n+1-j} - h \sum_{j=0}^{m} \beta_j \dot{\mathbf{u}}_{n+1-j} \Rightarrow \sum_{j=0}^{m} [\alpha_j \mathbf{I} - h \beta_j \mathbf{A}] \mathbf{u}_{n+1-j} = 0, \quad \alpha_0 = -1$$

- let $\{\mu_r\}_{r=1}^{r=n}$ be the eigenvalues of **A** and **X** be the matrix of associated eigenvectors $(\mathbf{X}^{-1}\mathbf{A}\mathbf{X} = \mathsf{diag}(\mu_1, \cdots, \mu_r, \cdots, \mu_n))$
- the characteristic equation associated with $\sum_{i=n}^{m} [\alpha_j \mathbf{I} h\beta_j \mathbf{A}] \mathbf{u}_{n+1-j} = 0$ is obtained by searching for a solution of the form

 $\mathbf{u}_{n+1-m} = \mathbf{X}\mathbf{a}$ (decomposition on an eigen basis) $\mathbf{u}_{(n+1-m)+1} = \lambda \mathbf{u}_{n+1-m} = \lambda \mathbf{X} \mathbf{a}$ (solution form) $\mathbf{u}_{n+1} = \lambda \mathbf{u}_n = \cdots = \lambda^k \mathbf{u}_{n+1-k} = \cdots = \lambda^m \mathbf{X} \mathbf{a}$

where $\lambda\in\mathbb{C}$ is called the solution amplification factor $(1,2) \in \mathbb{C}^{n} \times \mathbb{C}^{n}$

LStability Behavior of Numerical Solutions

 Analysis of the characteristic equation of a time-integration method (continue)

hence

$$\sum_{j=0}^{m} \left[\alpha_j \mathbf{I} - h \beta_j \mathbf{A} \right] \lambda^{m-j} \mathbf{X} \mathbf{a} = \mathbf{0}$$

■ since X⁻¹AX = diag(µ₁, · · · , µ_r, · · · , µ_n), premultiplying the above result by X⁻¹ leads to

$$\left[\sum_{j=0}^{m} \left[\alpha_{j}\mathbf{I} - h\beta_{j} \operatorname{diag}(\mu_{1}, \cdots, \mu_{r}, \cdots, \mu_{n})\right] \lambda^{m-j}\right] \mathbf{a} = \mathbf{0}$$

$$\implies \sum_{j=0}^{m} \left[\alpha_j - h \beta_j \mu_r \right] \lambda^{m-j} = 0, \ r = 1, 2, \cdots, n$$

• hence, the numerical response $\mathbf{u}_{n+1} = \lambda^m \mathbf{X} \mathbf{a}$ remains bounded if each solution of the above characteristic equation of degree m satisfies $|\lambda_k| < 1, \ k = 1, \cdots, m$

└─Stability Behavior of Numerical Solutions

- Analysis of the characteristic equation of a time-integration method (continue)
 - the stability limit is a circle of unit radius
 - in the complex plane of $\mu_r h$, the stability limit is therefore given by writing $\lambda = e^{i\theta}, 0 \le \theta \le 2\pi$

$$\implies \mu_r h = \frac{\sum_{j=0}^m \alpha_j e^{i(m-j)\theta}}{\sum_{j=0}^m \beta_j e^{i(m-j)\theta}}$$

■ one-step schemes (*m* = 1)

$$\mu_r h = \frac{\alpha_0 e^{i\theta} + \alpha_1}{\beta_0 e^{i\theta} + \beta_1} = \frac{-e^{i\theta} + \alpha_1}{\beta_0 e^{i\theta} + \beta_1}$$

└─Stability Behavior of Numerical Solutions

- Analysis of the characteristic equation of a time-integration method (continue)
 - one-step schemes (m = 1) (continue)

$$\mu_r h = \frac{\alpha_0 e^{i\theta} + \alpha_1}{\beta_0 e^{i\theta} + \beta_1} = \frac{-e^{i\theta} + \alpha_1}{\beta_0 e^{i\theta} + \beta_1}$$

- forward Euler: α₁ = 1, β₀ = 0, β₁ = −1 ⇒ μ_rh = e^{iθ} − 1 the solution is unstable in the entire plane except inside the circle of unit radius and center −1
- **backward Euler**: $\alpha_1 = 1$, $\beta_0 = -1$, $\beta_1 = 0 \Rightarrow \mu_r h = 1 e^{-i\theta}$ the solution is stable in the entire plane except inside the circle of unit radius and center 1
- $\frac{\text{trapezoidal rule: } \alpha_1 = 1, \ \beta_0 = -\frac{1}{2}, \ \beta_1 = -\frac{1}{2} \Rightarrow \mu_r h = \frac{2i\sin\theta}{1 + \cos\theta}$ the solution is stable in the entire left-hand plane

LStability Behavior of Numerical Solutions

 Analysis of the characteristic equation of a time-integration method (continue)

application to the single degree-of-freedom oscillator

$$\ddot{q} + \omega_0^2 q = 0, \qquad \mathbf{A} = \begin{bmatrix} 0 & -\omega_0^2 \\ 1 & 0 \end{bmatrix}$$

- the eigenvalues are $\mu_r = \pm i\omega_0$
- the roots $\mu_r h$ are located in the unstable region of the forward Euler scheme \Rightarrow amplification of the numerical solution
- the roots $\mu_r h$ are located in the stable region of the backward Euler scheme \Rightarrow decay of the numerical solution
- the roots $\mu_r h$ are located on the stable boundary of the trapezoidal rule scheme \Rightarrow the amplitude of the oscillations is preserved

└─The Newmark Method

Taylor's expansion of a function f

$$f(t_n + h) = f(t_n) + hf'(t_n) + \frac{h^2}{2}f''(t_n) + \dots + \frac{h^s}{s!}f^{(s)}(t_n) + \frac{1}{s!}\int_{t_n}^{t_n + h}f^{(s+1)}(\tau)(t_n + h - \tau)^s d\tau$$

Application to the velocities and displacements

$$f = \dot{\mathbf{q}}, s = 0 \quad \Rightarrow \quad \dot{\mathbf{q}}_{n+1} = \dot{\mathbf{q}}_n + \int_{t_n}^{t_{n+1}} \ddot{\mathbf{q}}(\tau) d\tau$$

$$f = \mathbf{q}, s = 1 \quad \Rightarrow \quad \mathbf{q}_{n+1} = \mathbf{q}_n + h \dot{\mathbf{q}}_n + \int_{t_n}^{t_{n+1}} \ddot{\mathbf{q}}(\tau) (t_{n+1} - \tau) d\tau$$
(2)

Given

 any approximation **q**(τ) of **q**(τ) in the time-interval [t_n, t_{n+1}] and any pair of quadrature rules for approximating the resulting integrals ∫_{t_n}<sup>t_{n+1} **q**(τ)dτ and ∫_{t_n}<sup>t_{n+1} **q**(τ)(t_{n+1} − τ)dτ

 or any pair of direct approximations of the time-integrals ∫_{t_n}<sup>t<sub>n+1</sup></sup> **q**(τ)dτ and ∫_{t_n}^{t_{n+1}} **q**(τ)(t_{n+1} − τ)dτ

 (9) leads to a numerical time-integration scheme for solving (1)

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L The Newmark Method

• Taylor expansions of $\ddot{\mathbf{q}}_n$ and $\ddot{\mathbf{q}}_{n+1}$ around $\tau \in [t_n, t_{n+1}]$

$$\ddot{\mathbf{q}}_n = \ddot{\mathbf{q}}(\tau) + \mathbf{q}^{(3)}(\tau)(t_n - \tau) + \mathbf{q}^{(4)}(\tau)\frac{(t_n - \tau)^2}{2} + \cdots$$
 (3)

$$\ddot{\mathbf{q}}_{n+1} = \ddot{\mathbf{q}}(\tau) + \mathbf{q}^{(3)}(\tau)(t_{n+1}-\tau) + \mathbf{q}^{(4)}(\tau)\frac{(t_{n+1}-\tau)^2}{2} + \cdots (4)$$

• Combine $(1 - \gamma)(3) + \gamma(4)$ and extract $\ddot{\mathbf{q}}(\tau)$

$$\implies \ddot{\mathbf{q}}(\tau) = (1-\gamma)\ddot{\mathbf{q}}_n + \gamma \ddot{\mathbf{q}}_{n+1} + \mathbf{q}^{(3)}(\tau)(\tau - h\gamma - t_n) + \mathcal{O}(h^2 \mathbf{q}^{(4)})$$

• Combine $(1 - 2\beta) (3) + 2\beta (4)$ and extract $\ddot{\mathbf{q}}(\tau)$

$$\implies \ddot{\mathbf{q}}(\tau) = (1 - 2\beta)\ddot{\mathbf{q}}_n + 2\beta\ddot{\mathbf{q}}_{n+1} + \mathbf{q}^{(3)}(\tau)(\tau - 2h\beta - t_n) + \mathcal{O}(h^2\mathbf{q}^{(4)})$$

L The Newmark Method

Substitute the 1st expression of
$$\ddot{\mathbf{q}}(\tau)$$
 in $\int_{t_n}^{t_{n+1}} \ddot{\mathbf{q}}(\tau) d\tau$

$$\implies \int_{t_n}^{t_{n+1}} \ddot{\mathbf{q}}(\tau) d\tau = \int_{t_n}^{t_{n+1}} \left((1-\gamma) \ddot{\mathbf{q}}_n + \gamma \ddot{\mathbf{q}}_{n+1} + \mathbf{q}^{(3)}(\tau) (\tau - h\gamma - t_n) + \mathcal{O}(h^2 \mathbf{q}^{(4)}) \right) d\tau \\ = (1-\gamma) h \ddot{\mathbf{q}}_n + \gamma h \ddot{\mathbf{q}}_{n+1} + \int_{t_n}^{t_{n+1}} \mathbf{q}^{(3)}(\tau) (\tau - h\gamma - t_n) d\tau + \mathcal{O}(h^3 \mathbf{q}^{(4)})$$

Apply the mean value theorem

$$\implies \int_{t_n}^{t_{n+1}} \ddot{\mathbf{q}}(\tau) d\tau = (1-\gamma)h \, \ddot{\mathbf{q}}_n + \gamma h \, \ddot{\mathbf{q}}_{n+1} + \mathbf{q}^{(3)}(\tilde{\tau}) \left[\frac{(\tau - h\gamma - t_n)^2}{2} \right]_{t_n}^{t_{n+1}} + \mathcal{O}(h^3 \mathbf{q}^{(4)})$$
$$= (1-\gamma)h \, \ddot{\mathbf{q}}_n + \gamma h \, \ddot{\mathbf{q}}_{n+1} + (\frac{1}{2} - \gamma)h^2 \mathbf{q}^{(3)}(\tilde{\tau}) + \mathcal{O}(h^3 \mathbf{q}^{(4)})$$

Substitute the 2nd expression of $\ddot{\mathbf{q}}(\tau)$ in $\int_{t_n}^{t_{n+1}} \ddot{\mathbf{q}}(\tau)(t_{n+1}-\tau)d\tau$

$$\Longrightarrow \int_{t_n}^{t_{n+1}} \ddot{\mathbf{q}}(\tau)(t_{n+1}-\tau)d\tau = (\frac{1}{2}-\beta)h^2\ddot{\mathbf{q}}_n + \beta h^2\ddot{\mathbf{q}}_{n+1} + (\frac{1}{6}-\beta)h^3\mathbf{q}^{(3)}(\tilde{\tau}) + \widetilde{\mathcal{O}}(h^4\ddot{\mathbf{q}}_{4)})$$

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$$= (1-\beta)h^2\ddot{\mathbf{q}}_n + \beta h^2\ddot{\mathbf{q}}_{n+1} + (1-\beta)h^2\dot{\mathbf{q}}_{n+1} + (1-\beta)h^2\dot{\mathbf{q$$

└─The Newmark Method

In summary

• $\forall \gamma$ and $\forall \beta$, the following holds true

$$\int_{t_n}^{t_{n+1}} \ddot{\mathsf{q}}(\tau) d\tau = (1-\gamma) h \, \ddot{\mathsf{q}}_n + \gamma h \, \ddot{\mathsf{q}}_{n+1} + \mathsf{r}_n$$

$$\int_{t_n}^{t_{n+1}} \ddot{\mathbf{q}}(\tau)(t_{n+1}-\tau) d\tau = \left(\frac{1}{2}-\beta\right) h^2 \ddot{\mathbf{q}}_n + \beta h^2 \ddot{\mathbf{q}}_{n+1} + \mathbf{r}'_n$$

where

$$\mathbf{r}_n = \left(\frac{1}{2} - \gamma\right) h^2 \mathbf{q}^{(3)}(\tilde{\tau}) + \mathcal{O}(h^3 \mathbf{q}^{(4)}); \quad \mathbf{r}'_n = \left(\frac{1}{6} - \beta\right) h^3 \mathbf{q}^{(3)}(\tilde{\tau}) + \mathcal{O}(h^4 \mathbf{q}^{(4)})$$

and $t_n < ilde{ au} < t_{n+1}$

neglecting each of r_n and r'_n on the ground that they are higher-order functions of the time-step h leads to the following family of time-integration schemes (Newmark's family) for solving (1)

$$\dot{\mathbf{q}}_{n+1} = \dot{\mathbf{q}}_n + (1-\gamma)h\,\ddot{\mathbf{q}}_n + \gamma h\,\ddot{\mathbf{q}}_{n+1} \tag{5}$$

$$\mathbf{q}_{n+1} = \mathbf{q}_n + h \dot{\mathbf{q}}_n + h^2 \left(\frac{1}{2} - \beta\right) \ddot{\mathbf{q}}_n + h^2 \beta \ddot{\mathbf{q}}_{n+1} \qquad (6)$$

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where γ and β are quadrature parameters

└─The Newmark Method

Particular values of the parameters γ and β • $\gamma = \frac{1}{2}$ and $\beta = \frac{1}{6}$ corresponds to linearly interpolating $\ddot{\mathbf{q}}(\tau)$ in $[t_n, t_{n+1}]$ $\ddot{\ddot{\mathbf{q}}}_{ln}(au) = \ddot{\mathbf{q}}_n + (au - t_n) \left(rac{\ddot{\mathbf{q}}_{n+1} - \ddot{\mathbf{q}}_n}{h}
ight)$ • $\gamma = \frac{1}{2}$ and $\beta = \frac{1}{4}$ corresponds to averaging $\ddot{\mathbf{q}}(\tau)$ in $[t_n, t_{n+1}]$ $\ddot{\ddot{\mathbf{q}}}_{av}(\tau) = \frac{\ddot{\mathbf{q}}_{n+1} + \mathbf{q}_n}{2}$ $\mathbf{q}(\tau)$ $- \ddot{\mathbf{q}}_{av}(\tau) = \frac{\ddot{\mathbf{q}}_{n+1} + \ddot{\mathbf{q}}_n}{2}$ $\ddot{\mathbf{q}}_{in}(\tau)$ ä tn tn+1

L The Newmark Method

- Application to the direct time-integration of $\mathbf{M}\ddot{\mathbf{q}} + \mathbf{C}\dot{\mathbf{q}} + \mathbf{K}\mathbf{q} = \mathbf{p}(t)$
 - write the equilibrium equation at t_{n+1} and substitute the expressions (5) and (6) into it

$$\implies [\mathbf{M} + \gamma h \mathbf{C} + \beta h^2 \mathbf{K}] \ddot{\mathbf{q}}_{n+1} = \mathbf{p}_{n+1} - \mathbf{C} [\dot{\mathbf{q}}_n + (1 - \gamma)h \ddot{\mathbf{q}}_n] \\ - \mathbf{K} \left[\mathbf{q}_n + h \dot{\mathbf{q}}_n + \left(\frac{1}{2} - \beta\right)h^2 \ddot{\mathbf{q}}_n \right]$$

- if the time-step *h* is uniform, $\mathbf{M} + \gamma h \mathbf{C} + \beta h^2 \mathbf{K}$ can be factored once solve the above system of equations for $\ddot{\mathbf{q}}_{n+1}$
- substitute the result into the expressions (5) and (6) to obtain $\dot{\mathbf{q}}_{n+1}$ and \mathbf{q}_{n+1}

Consistency of a Time-Integration Method

A time-integration scheme is said to be consistent if

$$\lim_{h\to 0}\frac{\mathbf{u}_{n+1}-\mathbf{u}_n}{h}=\dot{\mathbf{u}}(t_n)$$

The Newmark time-integration method is consistent

$$\lim_{h\to 0} \frac{\mathbf{u}_{n+1} - \mathbf{u}_n}{h} = \lim_{h\to 0} \left[\begin{array}{c} (1-\gamma)\ddot{\mathbf{q}}_n + \gamma \ddot{\mathbf{q}}_{n+1} \\ \dot{\mathbf{q}}_n + \left(\frac{1}{2} - \beta\right)h\ddot{\mathbf{q}}_n + \beta h\ddot{\mathbf{q}}_{n+1} \end{array} \right] = \left[\begin{array}{c} \ddot{\mathbf{q}}_n \\ \dot{\mathbf{q}}_n \end{array} \right]$$

Consistency is one necessary condition for convergence

└─Stability of a Time-Integration Method

- A time-integration scheme is said to be stable if there exists an integration time-step $h_0 > 0$ so that for any $h \in [0, h_0]$, a finite variation of the state vector at time t_n induces only a non-increasing variation of the state-vector \mathbf{u}_{n+j} calculated at a subsequent time t_{n+j}
- Stability is the other necessary condition for convergence

Stability of a Time-Integration Method

Premultiplying Eq. (5) and Eq. (6) by **M** and taking into account the equations of equilibrium (1) at t_n and t_{n+1} leads after some algebraic manipulations to

$$\begin{aligned} \mathbf{M}\dot{\mathbf{q}}_{n+1} &= \mathbf{M}\dot{\mathbf{q}}_n + h(1-\gamma)[-\mathbf{C}\dot{\mathbf{q}}_n - \mathbf{K}\mathbf{q}_n + \mathbf{p}_n] \\ &+ \gamma h[-\mathbf{C}\dot{\mathbf{q}}_{n+1} - \mathbf{K}\mathbf{q}_{n+1} + \mathbf{p}_{n+1}] \\ \mathbf{M}\mathbf{q}_{n+1} &= \mathbf{M}\mathbf{q}_n + h\mathbf{M}\dot{\mathbf{q}}_n + (\frac{1}{2} - \beta)h^2[-\mathbf{C}\dot{\mathbf{q}}_n - \mathbf{K}\mathbf{q}_n + \mathbf{p}_n] \\ &+ \beta h^2[-\mathbf{C}\dot{\mathbf{q}}_{n+1} - \mathbf{K}\mathbf{q}_{n+1} + \mathbf{p}_{n+1}] \end{aligned}$$
(7)

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Stability of a Time-Integration Method

Equations (7) can be re-written in matrix form as

$$\mathbf{u}_{n+1} = \mathbf{A}(h)\mathbf{u}_n + \mathbf{g}_{n+1}(h)$$

where $\boldsymbol{\mathsf{A}}$ is the amplification matrix associated with the integration operator

$$\mathbf{A}(h) = \mathbf{H}_{1}^{-1}(h)\mathbf{H}_{0}(h), \quad \mathbf{g}_{n+1} = \mathbf{H}_{1}^{-1}(h)\mathbf{b}_{n+1}(h)$$
$$\mathbf{b}_{n+1} = \begin{bmatrix} (1-\gamma)h\mathbf{p}_{n} + \gamma h\mathbf{p}_{n+1} \\ \left(\frac{1}{2} - \beta\right)h^{2}\mathbf{p}_{n} + \beta h^{2}\mathbf{p}_{n+1} \end{bmatrix}, \quad \mathbf{H}_{1} = \begin{bmatrix} \mathbf{M} + \gamma h\mathbf{C} & \gamma h\mathbf{K} \\ \beta h^{2}\mathbf{C} & \mathbf{M} + \beta h^{2}\mathbf{K} \end{bmatrix}$$
$$\mathbf{H}_{0} = -\begin{bmatrix} -\mathbf{M} + (1-\gamma)h\mathbf{C} & (1-\gamma)h\mathbf{K} \\ \left(\frac{1}{2} - \beta\right)h^{2}\mathbf{C} - h\mathbf{M} & -\mathbf{M} + \left(\frac{1}{2} - \beta\right)h^{2}\mathbf{K} \end{bmatrix}$$

└─Stability of a Time-Integration Method

Effect of an initial disturbance

$$\bullet \, \delta \mathbf{u}_0 = \mathbf{u}_0' - \mathbf{u}_0$$

$$\Longrightarrow \delta \mathbf{u}_{n+1} = \mathbf{A}(h) \delta \mathbf{u}_n = \mathbf{A}^2(h) \delta \mathbf{u}_{n-1} = \cdots = \mathbf{A}(h)^{n+1} \delta \mathbf{u}_0$$

• consider the eigenpairs of
$$A(h)$$

 $(\lambda_r, \mathbf{x}_r)$

then

$$\delta \mathbf{u}_{n+1} = \mathbf{A}^{n+1}(h) \sum_{s=1}^{2N} a_s \mathbf{x}_s = \sum_{s=1}^{2N} a_s \lambda_s^{n+1} \mathbf{x}_s$$

where \boldsymbol{N} is the dimension of the semi-discrete second-order dynamical system

 $\implies \delta \mathbf{u}_{n+1} \text{ will be amplified by the time-integration operator only if} the modulus of an eigenvalue of <math>\mathbf{A}(h)$ is greater than unity $\implies \delta \mathbf{u}_{n+1} \text{ will not be amplified by the time-integration operator if allow moduli of all eigenvalues of <math>\mathbf{A}(h)$ are less than unity

Stability of a Time-Integration Method

- Undamped case
 - decouple the equations of equilibrium by writing them (for the purpose of analysis) in the modal basis

$$\mathbf{q} = \mathbf{Q}\mathbf{y} = \sum_{i=1}^{N} y_i \mathbf{q}_{a_i} \Longrightarrow \ddot{y}_i + \omega_i^2 y_i = p_i(t)$$

apply the Newmark scheme to the *i*-th modal equation recalled above to obtain the amplification matrix

$$\mathbf{A}(h) = \begin{bmatrix} 1 - \gamma \frac{\omega_i^2 h^2}{1 + \beta \omega_i^2 h^2} & -\omega_i^2 h^2 \left(1 - \frac{\gamma}{2} \frac{\omega_i^2 h^2}{1 + \beta \omega_i^2 h^2} \right) \\ \frac{h}{1 + \beta \omega_i^2 h^2} & 1 - \frac{1}{2} \frac{\omega_i^2 h^2}{1 + \beta \omega_i^2 h^2} \end{bmatrix}$$

• characteristic equation is $\lambda^2 - \lambda \left(2 - (\gamma + \frac{1}{2})\eta^2\right) + 1 - (\gamma - \frac{1}{2})\eta^2 = 0$ where

$$\eta^2 = \frac{\omega_i^2 h^2}{1 + \beta \omega_i^2 h^2}$$

- characteristic equation has:
 - a pair of complex conjugate roots λ_1 and λ_2 if $\left(\gamma + \frac{1}{2}\right)^2 4\beta \leq \frac{4}{\omega_i^2 h^2} \Leftrightarrow \left(\gamma + \frac{1}{2}\right)^2 \eta^2 < 4, \quad i = 1, \dots, N \text{ (case 1)}$ two identical real roots if $\left(\gamma + \frac{1}{2}\right)^2 \eta^2 = 4 \text{ (case 2)}$ two distinct real roots if $\left(\gamma + \frac{1}{2}\right)^2 \eta^2 > 4 \text{ (case 3)}$

└─Stability of a Time-Integration Method

Undamped case (continue)

 \blacksquare it can be shown that case 1 is the limiting case, in which case

$$\lambda_{1,2} = \rho e^{\pm i\phi}$$

where

$$\begin{array}{lll} \rho & = & \sqrt{1 - \left(\gamma - \frac{1}{2}\right)\eta^2} \\ \\ \phi & = & \arctan\left(\frac{\eta\sqrt{1 - \frac{1}{4}(\gamma + \frac{1}{2})^2\eta^2}}{1 - \frac{1}{2}(\gamma + \frac{1}{2})\eta^2}\right) \end{array}$$

then, the Newmark scheme is stable if

$$ho \leq 1 \Rightarrow \gamma \geq rac{1}{2}$$

and

$$\left(\gamma+\frac{1}{2}\right)^2-4\beta\leq\frac{4}{\omega_i^2h^2},\quad i=1,\ \cdots,N$$

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 \implies limitation on the maximum time-step

└─Stability of a Time-Integration Method

- Undamped case (continue)
 - the algorithm is *conditionally* stable if

$$\gamma \geq \frac{1}{2}$$

• it is *unconditionally* stable if furthermore $\beta \geq \frac{1}{4}\left(\gamma + \frac{1}{2}\right)^2$ — that is,

$$\gamma \geq rac{1}{2} \quad ext{and} \quad eta \geq rac{1}{4} \left(\gamma + rac{1}{2}
ight)^2$$

• the choice $\gamma = \frac{1}{2}$ and $\beta = \frac{1}{4}$ leads to an unconditionally stable time-integration operator of maximum accuracy

Stability of a Time-Integration Method

Undamped case (continue)



└─Stability of a Time-Integration Method

■ Damped case (C ≠ 0)

- consider the case of modal damping
- then, the uncoupled equations of motion are

$$\ddot{y}_i + 2\xi_i\omega_i\dot{y}_i + \omega_i^2y_i = p_i(t)$$

where ξ_i is the modal damping coefficient

- consider the case $\gamma = \frac{1}{2}, \ \beta = \frac{1}{4}$
- an analysis similar to that performed in the undamped case reveals that in this case, the Newmark scheme remains stable as long as $\xi_i < 1$
- in general, damping has a stabilizing effect for moderate values of ξ_i

Amplitude and Periodicity Errors

Free-vibration of an undamped linear oscillator

$$\ddot{y} + \omega^2 y = 0$$
 and $y(0) = y_0, \ \dot{y}(0) = 0$ $\mathbf{A} = \begin{bmatrix} 0 & -\omega_0^2 \\ 1 & 0 \end{bmatrix}$

the above problem has an exact solution y(t) = y₀ cos ωt which can be written in complex discrete form as y_{n+1} = e^{iωh}y_n ⇒ the exact amplification factor is ρ_{ex} = 1 and the exact phase is φ_{ex} = ωh
 the numerical solution satisfies

$$\mathbf{u}_{n+1} = \begin{bmatrix} \dot{y}_{n+1} \\ y_{n+1} \end{bmatrix} = \mathbf{A}(h)\mathbf{u}_n$$

let
$$\lambda_{1,2}(\beta,\gamma)$$
 be the eigenvalues of $\mathbf{A}(h,\beta,\gamma)$
when $(\gamma + \frac{1}{2})^2 - 4\beta \leq \frac{4}{\omega_i^2 h^2}$, λ_1 and λ_2 are complex-conjugate
 $\lambda_{1,2}(\beta,\gamma) = \rho(\beta,\gamma)e^{\pm i\phi(\beta,\gamma)}$

where

$$\rho = \sqrt{1 - \left(\gamma - \frac{1}{2}\right)\eta^2}, \quad \phi = \arctan\left(\frac{\eta\sqrt{1 - \frac{1}{4}(\gamma + \frac{1}{2})^2\eta^2}}{1 - \frac{1}{2}(\gamma + \frac{1}{2})\eta^2}\right), \quad \eta^2 = \frac{\omega_*^2 h^2}{1 + \beta\omega^2 h^2}$$

Amplitude and Periodicity Errors

Free-vibration of an undamped linear oscillator (continue)
 amplitude error

$$ho-
ho_{\mathrm{ex}}=
ho-1=-rac{1}{2}\left(\gamma-rac{1}{2}
ight)\omega^2h^2+\mathcal{O}(h^4)$$

relative periodicity error

$$rac{\Delta T}{T} = rac{\Delta rac{1}{\phi}}{rac{1}{\phi}} = rac{rac{1}{\phi} - rac{1}{\phi_{\mathsf{ex}}}}{rac{1}{\phi_{\mathsf{ex}}}} = rac{\omega h}{\phi} - 1 = rac{1}{2} \left(eta - rac{1}{12}
ight) \omega^2 h^2 + \mathcal{O}(h^3)$$

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Amplitude and Periodicity Errors

Algorithm	γ	β	Stability limit	Amplitude error	Periodicity error ΔT
			ωπ	p - 1	T
Purely explicit	0	0	0	$\frac{\omega^2 h^2}{4}$	—
Central difference	$\frac{1}{2}$	0	2	0	$-\frac{\omega^2 h^2}{24}$
Fox & Goodwin	$\frac{1}{2}$	$\frac{1}{12}$	2.45	0	$\mathcal{O}(h^3)$
Linear acceleration	$\frac{1}{2}$	$\frac{1}{6}$	3.46	0	$\frac{\omega^2 h^2}{24}$
Average constant acceleration	$\frac{1}{2}$	$\frac{1}{4}$	∞	0	$\frac{\omega^2 h^2}{12}$

Table: Time-integration schemes of the Newmark family

- The purely explicit scheme ($\gamma = 0$, $\beta = 0$) is useless
- The Fox & Godwin scheme has asymptotically the smallest phase error but is only conditionally stable
- The average constant acceleration scheme $(\gamma = \frac{1}{2}, \beta = \frac{1}{4})$ is the unconditionally stable scheme with asymptotically the highest accuracy

└─Total Energy Conservation

Conservation of total energy

dynamic system with scleronomic constraints

$$rac{d}{dt}(\mathcal{T}+\mathcal{V})=-m\mathcal{D}+\sum_{s=1}^{n_s}Q_s\dot{q}_s$$

•
$$\mathcal{T} = \frac{1}{2} \dot{\mathbf{q}}^T \mathbf{M} \dot{\mathbf{q}}$$
 and $\mathcal{V} = \frac{1}{2} \mathbf{q}^T \mathbf{K} \mathbf{q}$

• the dissipation function $\tilde{\mathcal{D}}$ is a quadratic function of the velocities (m=2)

$$\mathcal{D} = \frac{1}{2} \dot{\mathbf{q}}^T \mathbf{C} \dot{\mathbf{q}}$$

external force component of the power balance

$$\sum_{s=1}^{n_s} Q_s \dot{q}_s = \dot{\mathbf{q}}^T \mathbf{p}$$

• integration over a time-step $[t_n, t_{n+1}]$

$$[\mathcal{T} + \mathcal{V}]_{t_n}^{t_{n+1}} = \int_{t_n}^{t_{n+1}} (-\dot{\mathbf{q}}^T \mathbf{C} \dot{\mathbf{q}} + \dot{\mathbf{q}}^T \mathbf{p}) dt$$

Total Energy Conservation

■ Conservation of total energy (continue)
 ■ note that because M and K are symmetric (M^T = M and K^T = K)

$$\begin{split} \left[\mathcal{T} + \mathcal{V}\right]_{t_n}^{t_{n+1}} &= \left[\mathcal{T}_{n+1} - \mathcal{T}_n\right] + \left[\mathcal{V}_{n+1} - \mathcal{V}_n\right] &= \frac{1}{2} (\dot{\mathbf{q}}_{n+1} - \dot{\mathbf{q}}_n)^T \mathbf{M} (\dot{\mathbf{q}}_{n+1} + \dot{\mathbf{q}}_n) \\ &+ \frac{1}{2} (\mathbf{q}_{n+1} - \mathbf{q}_n)^T \mathbf{K} (\mathbf{q}_{n+1} + \mathbf{q}_n) \end{split}$$

• when time-integration is performed using the Newmark algorithm with $\gamma = \frac{1}{2}$, $\beta = \frac{1}{4}$, the above variation becomes (see (5) and (6))

$$\left[\mathcal{T}+\mathcal{V}\right]_{t_n}^{t_{n+1}} = \frac{1}{2}(\mathbf{q}_{n+1}-\mathbf{q}_n)^{\mathsf{T}}(\mathbf{p}_n+\mathbf{p}_{n+1}) - \frac{h}{4}(\dot{\mathbf{q}}_{n+1}+\dot{\mathbf{q}}_n)^{\mathsf{T}}\mathbf{C}(\dot{\mathbf{q}}_{n+1}+\dot{\mathbf{q}}_n)$$

when applied to a conservative system (C = 0 and p = 0), preserves the total energy
 for non-conservative systems, [T + Y]^{tn+1}_{tn} = ∫^{tn+1}_{tp}(-q^TCq̇ + q̇^Tp)dt and therefore both terms in the right-hand side of the above formula result from numerical quadrature relationships that are consistent with the time-integration operator

$$\int_{t_n}^{t_{n+1}} \dot{\mathbf{q}}^T \mathbf{p} dt \approx \left(\int_{t_n}^{t_{n+1}} \dot{\mathbf{q}}^T dt\right) \left(\frac{\mathbf{p}_n + \mathbf{p}_{n+1}}{2}\right) = \frac{1}{2} (\mathbf{q}_{n+1} - \mathbf{q}_n)^T (\mathbf{p}_n + \mathbf{p}_{n+1})$$

$$\int_{t_n}^{t_{n+1}} \dot{\mathbf{q}}^T \mathbf{C} \dot{\mathbf{q}} dt \approx \left(\int_{t_n}^{t_{n+1}} \dot{\mathbf{q}}^T dt\right) \mathbf{C} \left(\frac{\dot{\mathbf{q}}_n + \dot{\mathbf{q}}_{n+1}}{2}\right) = \frac{1}{2} (\mathbf{q}_{n+1} - \mathbf{q}_n)^T \mathbf{C} \left(\frac{\dot{\mathbf{q}}_n + \dot{\mathbf{q}}_{n+1}}{2}\right)$$

$$= \frac{h}{4} (\dot{\mathbf{q}}_{n+1} + \dot{\mathbf{q}}_n)^T \mathbf{C} (\dot{\mathbf{q}}_{n+1} + \dot{\mathbf{q}}_n)$$

$$(1)$$

Explicit Time Integration Using the Central Difference Algorithm

Algorithm in Terms of Velocities

• Central Difference (CD) scheme = Newmark's with $\gamma = \frac{1}{2}$, $\beta = 0$

$$\dot{\mathbf{q}}_{n+1} = \dot{\mathbf{q}}_n + h_{n+1} \left(\frac{\ddot{\mathbf{q}}_n + \ddot{\mathbf{q}}_{n+1}}{2} \right)$$

$$\mathbf{q}_{n+1} = \mathbf{q}_n + h_{n+1} \dot{\mathbf{q}}_n + \frac{h_{n+1}^2}{2} \ddot{\mathbf{q}}_n$$
(8)

where $h_{n+1} = t_{n+1} - t_n$ Equivalent three-step form

start with

$$\mathbf{q}_{n} = \mathbf{q}_{n-1} + h_{n} \dot{\mathbf{q}}_{n-1} + \frac{h_{n}^{2}}{2} \ddot{\mathbf{q}}_{n-1} = \mathbf{q}_{n-1} + h_{n} \underbrace{\left(\dot{\mathbf{q}}_{n-1} + \frac{h_{n}}{2} \ddot{\mathbf{q}}_{n-1} \right)}_{\dot{\mathbf{q}}_{n-\frac{1}{2}}} \quad (9)$$

divide by h_n and subtract the result from q_{n+1} divided by h_{n+1}
 account for the relationship (8)

$$\implies \ddot{\mathbf{q}}_{n} = \frac{h_{n}(\mathbf{q}_{n+1} - \mathbf{q}_{n}) - h_{n+1}(\mathbf{q}_{n} - \mathbf{q}_{n-1})}{h_{n+\frac{1}{2}}h_{n}h_{n+1}} \tag{10}$$

where $h_{n+\frac{1}{2}} = \frac{h_{n} + h_{n+1}}{2}$

-Explicit Time Integration Using the Central Difference Algorithm

└─Algorithm in Terms of Velocities

Case of a constant time-step h

$$\ddot{\mathbf{q}}_n = \frac{\mathbf{q}_{n+1} - 2\mathbf{q}_n + \mathbf{q}_{n-1}}{h^2}$$

- Efficient implementation
 - use a lumped mass matrix M
 - **i** initialize: $\ddot{\mathbf{q}}_0 = \mathbf{M}^{-1}(\mathbf{p}_0 \mathbf{K}\mathbf{q}_0)$ and $\dot{\mathbf{q}}_{\frac{1}{2}} = \dot{\mathbf{q}}_0 + \frac{h_1}{2}\ddot{\mathbf{q}}_0$
 - **i** increment the displacement: $\mathbf{q}_n = \mathbf{q}_{n-1} + h_n \dot{\mathbf{q}}_{n-\frac{1}{2}}$ (see (9))
 - **compute the acceleration:** $\ddot{\mathbf{q}}_n = \mathbf{M}^{-1}(\mathbf{p}_n \mathbf{K}\mathbf{q}_n)$ (enforce equilibrium at t_n)
 - increment the velocity at half time-step (formula results from (10))

$$\dot{\mathbf{q}}_{n+\frac{1}{2}} = \dot{\mathbf{q}}_{n-\frac{1}{2}} + h_{n+\frac{1}{2}}\ddot{\mathbf{q}}_{n} \Leftrightarrow \ddot{\mathbf{q}}_{n} = \frac{\dot{\mathbf{q}}_{n+\frac{1}{2}} - \dot{\mathbf{q}}_{n-\frac{1}{2}}}{h_{n+\frac{1}{2}}}$$

- Stability condition: for $\gamma = 1/2$ and $\beta = 0$, $\left(\gamma + \frac{1}{2}\right)^2 4\beta \leq \frac{4}{\omega_{cr}^2 h^2} \Rightarrow \omega_{cr} h \leq 2$ where ω_{cr} is the highest frequency contained in the model this condition is also known as the *Courant condition*
- $h_{cr} = \frac{2}{\omega_{cr}}$ is referred to here as the maximum Courant stability time-step

-Explicit Time Integration Using the Central Difference Algorithm

Application Example: the Clamped-Free Bar Excited by an End Load

- Clamped bar subjected to a step load at its free end
- Model made of N = 20 finite elements with equal length $I = \frac{L}{N}$



- Iumped mass matrix
- Eigenfrequencies of the continuous system

$$\omega_{cont_r} = (2r-1)\frac{\pi}{2}\sqrt{\frac{EA}{mL^2}} = \left(\frac{2r-1}{N}\right)\frac{\pi}{2}\sqrt{\frac{EA}{ml^2}} = \left(\frac{2r-1}{N}\right)\frac{\pi}{2}$$

Explicit Time Integration Using the Central Difference Algorithm

Application Example: the Clamped-Free Bar Excited by an End Load

Finite element stiffness and mass matrices

Analytical frequencies of the discrete system

$$\begin{split} \omega_r &= 2\sqrt{\frac{EA}{ml^2}}\sin\left(\left(\frac{2r-1}{2N}\right)\frac{\pi}{2}\right) &= 2\sin\left(\left(\frac{2r-1}{2N}\right)\frac{\pi}{2}\right), \quad r = 1, 2, \ \cdots N\\ &\Rightarrow \quad \omega_{cr} < \omega_{cr}(r = N, N \to \infty) = 2 \end{split}$$

Critical time-step for the CD algorithm

$$\omega_{cr}h_{cr} = 2 \Rightarrow h_{cr} = 1$$

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Explicit Time Integration Using the Central Difference Algorithm

└─Application Example: the Clamped-Free Bar Excited by an End Load



Explicit Time Integration Using the Central Difference Algorithm

Application Example: the Clamped-Free Bar Excited by an End Load



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Restitution of the Exact Solution by the Central Difference Method

- For the clamped-free bar example, the CD method computes the exact solution when $h = h_{cr}$
- Comparison of the exact solution of the continuous free-vibration bar problem and the analytical expression of the numerical solution
 - denote by $q_{j,n}$ the value of the *j*-th d.o.f. at time t_n
 - if $q_{j,n}$ is not located at the boundary, it satisfies (see (11))

$$\frac{ml}{h^2}(q_{j,n+1}-2q_{j,n}+q_{j,n-1})+\frac{EA}{l}(-q_{j-1,n}+2q_{j,n}-q_{j+1,n})=0$$

the general solution of the above problem is

$$q_{j,n} = \underbrace{\sin(j\mu + \phi)}_{\text{spatial component}} \underbrace{[a \cos n\theta + b \sin n\theta]}_{\text{temporal component}}$$
(12)

 comparing the above expression to the exact harmonic solution of the continuous form of this free-vibration problem (which can be derived analytically)

$$\implies n\theta = \omega t = \omega nh \Rightarrow \frac{\theta}{h} = \omega_{num}$$

-Explicit Time Integration Using the Central Difference Algorithm

Restitution of the Exact Solution by the Central Difference Method

- Comparison of the exact solution of the free-vibration bar problem and the analytical expression of the numerical solution (continue)
 - introduce the exact expression for $q_{j,n}$ in the CD scheme

$$2[(1-\cos\mu)-\lambda^2(1-\cos\theta)]q_{j,n}=0$$

where
$$\lambda^2 = \left(\frac{ml^2}{EA}\right) \frac{1}{h^2} = \frac{1}{h^2} \Rightarrow 1 - \cos\theta = \frac{1}{\lambda^2} (1 - \cos\mu)$$

■ make use of the boundary conditions in space $(q_{0,n} = 0, \text{ and plug}$ (12) in the last equation in (11)) $\implies \phi = 0 \text{ and } \mu_r = \left(\frac{2r-1}{N}\right) \frac{\pi}{2}, r \in \mathbb{N}^*$

$$\Longrightarrow 1 - \cos \theta_r = \frac{1}{\lambda^2} (1 - \cos \mu_r)$$

• special case $\lambda^2 = 1$ $(h = h_{cr} = 1) \Rightarrow \theta_r = \mu_r$ and

$$\omega_{\textit{num}_{r}} = \frac{\theta_{r}}{h} = \mu_{r} = \left(\frac{2r-1}{N}\right) \frac{\pi}{2} \sqrt{\frac{EA}{ml^{2}}} = \left(\frac{2r-1}{N}\right) \frac{\pi}{2}$$

 \implies the *r*-th numerical frequency coincides with the *r*-th eigenfrequency of the continuous system